NOVEMBER 22, 2024

PROJECT NO: 624-6777

SENT VIA: EMAIL

TOWN OF CALEDON PLANNING RECEIVED Dec 17, 2024

Toronto and Region Conservation Authority 101 Exchange Avenue Vaughan, ON L4K 5R6

Attention: Michael Hynes, MES, MCIP, RPP Senior Planner, Development Planning and Permits

RE: PROLOGIS HUMBER STATION – TEMPORARY STORMWATER MANAGEMENT OUTLET

Dear Michael,

As discussed in our meeting on Friday, September 6th, 2024, C.F. Crozier & Associates Inc. (Crozier) was retained by Prologis to prepare a detailed stormwater management design to support the Site Plan Application for a proposed industrial development located at 12519-12713 Humber Station Road in the Town of Caledon. The property is located within the Humber Station Villages Employment Area, Lots 1-5, Concession 5 (Albion) in the Town of Caledon. A Comprehensive Environmental Impact Study and Management Plan (CEISMP) for Phase 1 dated July 2024 and Phase 2 dated November 2024 were prepared by Schaeffers to support the development of Humber Station Villages. The management plan includes roads, storm sewers and a pond downstream of the Prologis property which will ultimately serve as the outlet for the development. As the parcel (12285 Humber Station Road) north of the SWM Block is a non-participating landowner, it is unclear when the proposed road and storm sewer will be designed and constructed.

This letter has been prepared to support a temporary outlet to the Clarkway Tributary for the first phase of the Prologis development which would remain in place until the downstream storm sewers and stormwater management pond proposed in the CEISMP are constructed. The terms of reference for this analysis were confirmed through email correspondence between Crozier and the TRCA dated September 19, 2024. A copy of this email is attached for reference.

The first phase of the Prologis development (Phase 1A) occupies approximately 27.6 ha in the northeast of the site. Refer to the Master Site Plan (Petroff, November 14, 2024) and Drawing C200 for the General Servicing Plan illustrating the proposed development. Most of Phase 1A drains to the existing HDF 8 just south of the property. This feature is not a suitable outlet for the development as it is not a defined drainage feature. Based on the concept plan layout, we would like to propose an outlet towards the Clarkway Tributary, at the southeast corner of the property. We have completed the following analysis to support the flow diversion and interim outlet location based on correspondence with the TRCA.

1. The initial step is to calculate the required site release rates for the portion of the subject properties that drains to the Clarkway Tributary under existing conditions and determine the storage requirements for the stormwater management (SWM) pond accordingly using the criterion.





The Humber River unitary flow rates for Sub-basin 36 were used to determine the allowable release rates to the Clarkway Tributary for the property. The 9.56 ha area along the northeast property line that drains to the Clarkway Tributary in existing conditions was used for this calculation. The TRCA Humber catchments were used to delineate this area as shown in **Figure 1**.

The storage required to control Phase 1A to the unitary release rates was determined using VO and the TRCA 12-hour AES storm events since it produced the highest peak flows and storage values in the Phase 1A model. The 100-year release rate was calculated to be 0.236 m³/s. To control Phase 1A to this release rate, a storage of 18,385 m³ is required for the 100-year event and a storage of 43,358 m³ is required for the Regional event. Refer to **Figure 2** for the post-development drainage area plan and **Table 1** below for calculated release rates and required storage. Detailed model parameters are provided as an attachment to this letter, as is a copy of the VO model.

Return Period	Calculated U	Init Flow Rate	Required Storage
	L/s	(m³/s)	m ³
2-Year	75.4	0.075	8,898
5-Year	115.6	0.116	11,525
10-Year	142.0	0.142	13,156
25-Year	178.9	0.179	15,308
50-Year	209.0	0.209	16,838
100-Year	236.0	0.236	18,385
Regional	678.8	0.679	43,358

Table 1: R	elease Rates	and	Storage
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2. Identify and describe the constraint that makes the above requirement challenging, and propose an approach to resolve the issue without negatively affecting the receiving feature (flooding and erosion).

In order to control the Phase 1A area to the relatively small target release rates for the Clarkway Tributary as described above, a significant amount of storage is required for the interim outlet conditions. There is insufficient space available within the property to provide a SWM facility large enough to contain this quantity of storage. Therefore, a sensitivity analysis has been completed using the TRCA Humber VO model to determine the maximum discharge from Phase 1A that does not impact the conditions downstream of the property. Further details on the analysis are provided below.

- 3. Identify contributing drainage area to the proposed interim outlet location in Clarkway Tributary and summarize in a drainage map.
 - a. Compare the upstream area and diversion area.

Figure 2 attached shows the post-development catchment areas from Phase 1A that are to drain to the interim outlet location. It also shows the proposed SWM facility. **Figure 3** shows the upstream area contributing to the tributary and highlights the areas within Phase 1A that are proposed to be diverted. The diverted areas are summarized in the table below. As shown, the 13.93 ha diverted area is very minor in comparison to the total 651.6 ha contributing area, representing 2.14% of the total area.

	lable	2: Diverted Ar	ea summary	
TRCA VO Catchment	Drainage Feature	Diverted Area (ha)	Total Upstream Area (ha)	Diverted Area Percentage (%)
43.03	HDF-8	13.93	651.60	2.14

Table 2. Diverted Area Summary

b. Compare the modeled flows from the diversion area to the modeled flows in the Clarkway Tributary.

The unitary release rate for Phase 1A has been calculated as described in Comment 1 above. The table below compares the Phase 1A peak flow with the total peak flow at the Phase 1A outlet location (J124) for the 2 to 100-year storm events. The target release rate has been area weighted for the diverted area. As shown in the table below, the 100-year release rate for the diverted area represents approximately 0.2% of the total peak flow within the Clarkway Tributary at the Phase 1A outlet location and the Regional represents 0.43%.

Storm Event	Phase 1A Release Rate (m ³ /s)	Area Weighted Peak Flow for Diverted Area (m ³ /s)	J124 Peak Flow (m³/s)	Diverted Area / J124 Peak Flow Percentage (%)
2 Yr 6 Hour AES	0.075	0.025	10.489	0.24
5 Yr 6 Hour AES	0.116	0.039	16.855	0.23
10 Yr 6 Hour AES	0.142	0.048	21.382	0.22
25 Yr 6 Hour AES	0.179	0.060	28.307	0.21
50 Yr 6 Hour AES	0.209	0.070	33.397	0.21
100 Yr 6 Hour AES	0.236	0.079	39.218	0.20
Regional	0.679	0.332	77.841	0.43

Table 3: Peak Flow Comparison

- 4. Conduct flow comparison and sensitivity analysis in the VO model regarding flow diversion for the Clarkway Tributary.
 - a. Review and assess the downstream impact assuming the flow change and no SWM controls.
 - i. Assess the flow change within the Clarkway Tributary at the proposed interim outlet location.

A sensitivity analysis was completed by adding the Phase 1A post-development catchments into the TRCA Humber VO model and adjusting the existing catchments accordingly. The Phase 1A catchments were routed to the Clarkway Tributary outlet location without storage so the impacts on the downstream system could be analyzed. The 6-hour AES storm was used for this analysis as it produces the highest peak flows in the TRCA Humber VO model. The table below summarizes the results at the outlet location (J124) and two other locations farther downstream (J4200.683 and J1700.594). Detailed results are attached for reference.

	Table 4: Teak Hor	v kelease kates w	iniour storage	
Storm Event	Junction	Pre-dev Peak Flow Rate (m³/s)	Post-dev Peak Flow Rate (m³/s)	Percent Increase (%)
	J124	10.489	10.531	0.4
2 Yr 6 Hr AES	J4200.683	10.618	10.651	0.3
	J1700.594	10.510	10.589	0.8
	J124	16.855	16.891	0.2
5 Yr 6 Hr AES	J4200.683	17.191	17.253	0.4
	J1700.594	16.180	16.289	0.7
	J124	21.382	21.385	0.0
10 Yr 6 Hr AES	J4200.683	21.659	21.767	0.5
	J1700.594	20.046	20.214	0.8
	J124	23.307	28.374	0.2
25 Yr 6 Hr AES	J4200.683	27.434	27.711	1.0
	J1700.594	25.359	25.563	0.8
	J124	33.397	33.503	0.3
50 Yr 6 Hr AES	J4200.683	33.683	33.913	0.7
	J1700.594	29.487	29.736	0.8
	J124	39.218	39.106	-0.3
100 Yr 6 Hr	J4200.683	39.415	39.739	0.8
	J1700.594	34.201	34.558	1.0
	J124	76.143	77.841	2.2
Hazel	J4200.683	90.288	92.357	2.3
	J1700.594	110.616	112.683	1.9

Table 4: Peak Flow Release Rates without Storage

ii. Assess the flow change within the Clarkway Tributary downstream of the proposed outlet to the confluence at node 43.2 (downstream of Mayfield Road).

Node 43.2 is located at J124 within the VO model. See results for J124 presented in the table above.

- b. Complete sensitivity analysis for the following:
 - i. Required Site release rate calculated based on unitary release rate calculated using the existing drainage area to the Clarkway Tributary.

A sensitivity analysis was completed by adding the Phase 1A post-development catchments into the TRCA Humber VO model and adjusting the existing catchments accordingly. The Phase 1A catchments were routed to a reservoir so the Phase 1A peak flow discharging to the Clarkway Tributary could be iterated and the impacts on the downstream system could be analyzed. The first scenario included setting the Phase 1A discharge according to the unitary flow rate calculations. Then, the peak flow was increased and the impacts on the peak flows were analyzed.

The modeling shows that if the unitary release rate for the 2-to-100-year storm events is multiplied by 4, the impacts to downstream peak flows are negligible. The 6-hour AES storm events were used for the analysis. Similarly, if the regional release rate is multiplied by 2.5, the impacts to downstream peak flows are negligible. The table below summarize the results.

Storm Event	Phase 1A Release Rate (m³/s)	Phase 1A Storage (m ³)	Junction	Pre-dev Peak Flow Rate (m ³ /s)	Post-dev Peak Flow Rate (m ³ /s)	Percent Increase (%)
			J124	10.489	10.450	-0.4
2 Yr	0.300	0.705	J4200.683	10.618	10.641	0.2
			J1700.594	10.510	10.556	0.4
			J124	16.855	16.823	-0.2
5 Yr	0.464	0.921	J4200.683	17.191	17.223	0.2
			J1700.594	16.180	16.227	0.3
			J124	21.382	21.314	-0.3
10 Yr	0.568	1.071	J4200.683	21.659	21.714	0.3
			J1700.594	20.046	20.120	0.4
			J124	23.307	27.877	-1.5
25 Yr	0.716	1.248	J4200.683	27.434	27.523	0.3
			J1700.594	25.359	25.465	0.4
			J124	33.397	33.293	-0.3
50 Yr	0.836	1.387	J4200.683	33.683	33.573	-0.3
			J1700.594	29.487	29.603	0.4
			J124	39.218	39.105	-0.3
100 Yr	0.944	1.513	J4200.683	39.415	39.403	0.0
			J1700.594	34.201	34.305	0.3
			J124	76.143	76.101	-0.1
Hazel	1.697	3.362	J4200.683	90.288	90.246	0.0
			J1700.594	110.616	110.594	0.0

Table 5: Pe	eak Flow	Release	Rates with	Storage

ii. Unit flow rates calculated using the area for the existing areas draining to Clarkway Tributary and the area to be diverted from HDF 8.

The unitary flow rates based on the existing areas draining to the Clarkway Tributary per the CEISMP report, are summarized in the table below.

Return Period	Calculated Unit Flow Rate			
kelom renou	L/s	(m³/s)		
2-Year	192.3	0.192		
5-Year	292.9	0.293		
10-Year	361.4	0.361		
25-Year	454.3	0.454		
50-Year	528.4	0.528		
100-Year	598.4	0.598		
Regional	3,362	3.362		

Table 6: Unitary Release Rates

A scenario was created and run in the TRCA Humber VO model with Phase 1A discharging these peak flows to the Clarkway Tributary and the downstream nodes were analyzed. The results are summarized in the table below.

Storm Event	Junction	Pre-dev Peak Flow Rate (m³/s)	Post-dev Peak Flow Rate (m ³ /s)	Percent Increase (%)
	J124	10.489	10.311	-1.7
2 Yr	J4200.683	10.618	10.505	-1.1
	J1700.594	10.510	10.448	-0.6
	J124	16.855	16.592	-1.6
5 Yr	J4200.683	17.191	16.998	-1.1
	J1700.594	16.180	16.075	-0.6
	J124	21.382	21.053	-1.5
10 Yr	J4200.683	21.659	21.444	-1.0
	J1700.594	20.046	19.912	-0.7
	J124	23.307	27.588	-2.5
25 Yr	J4200.683	27.434	27.202	-0.8
	J1700.594	25.359	25.218	-0.6
	J124	33.397	32.952	-1.3
50 Yr	J4200.683	33.683	33.253	-1.3
	J1700.594	29.487	29.305	-0.6
	J124	39.218	38.707	-1.3
100 Yr	J4200.683	39.415	39.008	-1.0
	J1700.594	34.201	33.911	-0.8
	J124	76.143	75.769	-0.5
Hazel	J4200.683	90.288	89.917	-0.4
	J1700.594	110.616	110.332	-0.3

Table 7: Peak Flow Analysis Summary

Please note, the detailed Phase 1A model that was used for this analysis has been revised since the initial analysis described in 4a above was completed. Therefore, there are slight discrepancies in the VO model parameter inputs, but the conclusions remain the same. Please refer to the Stormwater Management Implementation Report prepared by Crozier for further information regarding the detailed design VO model parameters. iii. Proposed Site release rate to Clarkway Tributary (rate calculated in bullet ii reduced by the percentage required to produce no significant downstream impacts.

As discussed in Comment 4b above, multiplying the unitary release rates for the Phase 1A by 4 for the 2- to 100-year and 2.5 for the Regional storm event has a negligible impact on the downstream system and requires the smallest amount of additional site storage. These release rates are higher than those required for the ultimate conditions for the 2 through 100 year storm events. As the Prologis development needs to be designed to meet the release rates identified in the CEISMP, the storage and controls for the 2-100 year storms are provided within the Phase 1A development area. These release rates are shown in green in the table below. The release rate calculated for the regional storm is smaller under interim conditions than the release rate for the ultimate conditions, so additional storage will be required.

Therefore, we are proposing the following release rates, shown in green, for Phase 1A to the temporary outlet.

	Phase 1A Re		Storage R	•
Storm Event	(m ³ /	(s)	(m	3)
	Interim	Phase 1A	Phase 1A	Interim Pond
2 Yr	0.300	1920	0.705	6516
5 Yr	0.464	2930	0.921	8077
10 yr	0.568	3610	1.071	8822
25 Yr	0.716	4540	1.248	9774
50 Yr	0.836	5280	1.387	10193
100 Yr	0.944	5980	1.513	10636
Regional	1.697	22270	3.362	16540

Table 8: Proposed Phase 1A Release Rates

- 5. Conduct hydraulic downstream impact assessment.
 - a. Update the HEC-RAS model with the updated flow from task #2.
 - b. Evaluate the impact on water surface elevations and velocity between the proposed interim outlet location and node 43.2 (Mayfield Road).

The sensitivity analysis shows that if the release rates from 4 ii. are used as described above, there are no increases in downstream peak flows. The node directly downstream (J124) has a percent decrease of -0.8% for the 100-year storm event. Since there are no increases in peak flows, the downstream watercourse will not be impacted and no changes to the HEC-RAS model are required.

- 1. Erosion Assessment
 - a. Fluvial engineer to review the erosion impact at the outlet location and points of interest downstream of the Site.
 - b. Since diverting areas will lead to a significant increase in runoff volume, which may cause instream erosion, it is crucial that the stormwater management (SWM) strategy includes a minimum of 10mm of onsite runoff retention, which can be managed through infiltration or evapotranspiration.
 - c. 48-hour drawdown time is required for the SWM pond.

A minimum of 10 mm of onsite retention has been accounted for in the design and 48 hours of extended detention will be provided by the interim pond. Please refer to the Stormwater Management Implementation Report prepared by Crozier under a separate cover for further details regarding erosion and water balance.

- 2. Water balance
 - a. Site specific water balance should be maintained, matching post-development infiltration volume to pre-development.
 - b. Fill loading impact on soil infiltration, mounding and LID infiltration rates should be reviewed by a geotechnical and hydrogeological engineer.

Comment 2 is addressed in the Stormwater Management Implementation Report prepared by Crozier under a separate cover. Please refer to this report for further details regarding water balance.

The analysis and responses described in responses to comments 1 to 5 above demonstrate that Phase 1A of the development can discharge to the Clarkway Tributary without negatively impacting the conditions downstream. Therefore, we believe that using the Clarkway Tributary as a temporary outlet during interim conditions is the best solution.

Should you have any questions or require any further information, please do not hesitate to contact the undersigned.

Sincerely,

C.F. CROZIER & ASSOCIATES INC.

Maggie Findlay, EIT Land Development

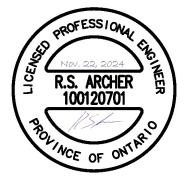
MF/tc

C.C.

Enclosure Email correspondence Hydrologic Input Parameters Unitary Release Rate Calculations Diverted Area Summary VO Model Sensitivity Analysis Output Tables Figure 1 - Interim Conditions Target Release Rate Figure 2 - Interim Conditions Drainage Figure Figure 3 - Interim Conditions Diverted Drainage Areas

C.F. CROZIER & ASSOCIATES INC.

Rebecca Archer, P.Eng. Senior Project Engineer



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From: Michael Hynes <<u>Michael.Hynes@trca.ca</u>> Sent: Thursday, September 19, 2024 12:28 PM To: Hamdy Shafi <<u>hshafi@cfcrozier.ca</u>>; Dilnesaw Chekol <<u>Dilnesaw.Chekol@trca.ca</u>> Cc: Heaven Lin <<u>hlin@cfcrozier.ca</u>>; Rebecca Archer <<u>rarcher@cfcrozier.ca</u>>; Mena Iskander <<u>miskander@cfcrozier.ca</u>>; Canejo, Carlos <<u>ccanejo@prologis.com</u>>; Joe Plutino <<u>jplutino@mainlineplanning.com</u>>; Jim Davidson <<u>Jdavidson@mainlineplanning.com</u>>; Adam Miller <<u>Adam.Miller@trca.ca</u>>; Jason Wagler <<u>Jason.Wagler@trca.ca</u>>; Dilnesaw Chekol <<u>Dilnesaw.Chekol@trca.ca</u>>; Ali Shirazi <<u>Ali.Shirazi@trca.ca</u>> Subject: RE: Prologis Humber Station - SWM Outlet Discussion with TRCA

Good Afternoon,

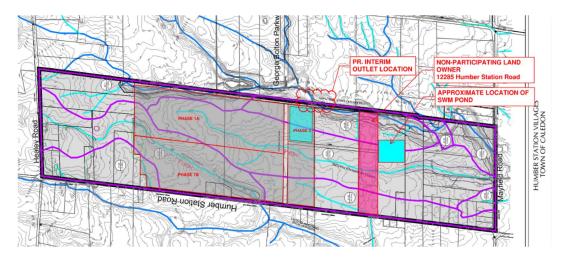
Technical staff have reviewed your Prologis Humber Station - SWM Outlet Discussion with TRCA and would provide the following changes in red below. Sorry for the delay in responding. Should you have questions please contact me.

Good Morning Michael,

Thank you for meeting with us on Friday, September 6, 2024. As discussed, please see the meeting summary below and our inquiries regarding the SWM outlet.

We have completed a submission dated April 2024 for the Draft Plan of Subdivision, Zoning Bylaw Amendment, and Site Plan Application following CEISMP Phase 1. Due to the delay in the CEISMP Phase 2 submission, the Town has not started their review, or circulated documents to agencies for formal review. Please see attached the Site Plan for context of the proposed development (Phase 1 of the Prologis Humber Station Distribution Centre).

Following the design concept of CEISMP Phase 1 and 2, an end-of-pipe SWM facility is proposed to provide water quantity and quality control for the Humber Station Employment area. As the parcel (12285 Humber Station Road) north of the SWM Block is a non-participating landowner, it is unclear when the proposed road and storm sewer will be designed and constructed. This may delay the Prologis Humber Station construction schedule. Therefore, we would like to explore the SWM outlet location as an interim solution.



The Site is draining to the existing HDF 8 just south of the Site. This feature is not a suitable outlet for the development as it is not a defined drainage feature. Based on the concept plan layout, we would like to propose an outlet towards the Clarkway Tributary, at the southeast corner of the Site. Based on the meeting discussion we will provide the following analysis to support the flow diversion and interim outlet location:

- 1. The initial step is to calculate the required site release rates for the portion of the subject properties that drains to the Clarkway Tributary under existing conditions and determine the storage requirements fo the stormwater management (SWM) pond accordingly using the criterion.
- 2. Identify and describe the constraint that makes the above requirement challenging, and propose an approach to resolve the issue without negatively affecting the receiving feature (flooding and erosion).
- 3. Identify contributing drainage area to the proposed interim outlet location in Clarkway Tributary and summarize in a drainage map.
 - a. Compare the upstream area and diversion area.
 - b. Compare the modeled flows from the diversion area to the modeled flows in the Clarkway Tributary.
- 4. Conduct flow comparison and sensitivity analysis in the VO model regarding flow diversion for the Clarkway Tributary.
 - a. Review and assess the downstream impact assuming the flow change and no SWM controls.
 - i. Assess the flow change within the Clarkway Tributary at the proposed interim outlet location.
 - ii. Assess the flow change within the Clarkway Tributary downstream of the proposed outlet to the confluence at node 43.2 (downstream of Mayfield Road).
 - b. Complete sensitivity analysis for the following:
 - i. Required Site release rate calculated based on unitary release rate calculated using the existing drainage area to the Clarkway Tributary.
 - ii. Unit flow rates calculated using the area for the existing areas draining to Clarkway Tributary and the area to be diverted from HDF 8.
 - iii. Proposed Site release rate to Clarkway Tributary (rate calculated in bullet ii reduced by the percentage required to produce no significant downstream impacts.
- 5. Conduct hydraulic downstream impact assessment.
 - a. Update the HEC-RAS model with the updated flow from task #2.
 - b. Evaluate the impact on water surface elevations and velocity between the proposed interim outlet location and node 43.2 (Mayfield Road).

The above tasks (1, 2, and 3) will be summarized in a memorandum for TRCA's review. Additional topics such as erosion and water balance criteria were also discussed in the meeting, and these criteria will be incorporated into the submission for Draft Plan of Subdivision, Zoning Bylaw Amendment, and Site Plan Application.

1. Erosion Assessment

- a. Fluvial engineer to review the erosion impact at the outlet location and points of interest downstream of the Site.
- b. Since diverting areas will lead to a significant increase in runoff volume, which may cause instream erosion, it is crucial that the stormwater management (SWM) strategy includes a minimum of 10mm of onsite runoff retention, which can be managed through infiltration or evapotranspiration.
- c. 48-hour drawdown time is required for the SWM pond.
- 2. Water balance
 - a. Site specific water balance should be maintained, matching post-development infiltration volume to pre-development.
 - b. Fill loading impact on soil infiltration, mounding and LID infiltration rates should be reviewed by a geotechnical and hydrogeological engineer.

Please advise if our understanding of the above is correct. We look forward to providing more information to begin the TRCA's preliminary review and advancement of the Prologis Humber Station site design.

Thanks, Hamdy

Michael Hynes, MES, MCIP, RPP

Senior Planner Development Planning and Permits | Development and Engineering Services

T: (437) 880-2327 E: <u>michael.hynes@trca.ca</u> A: 101 Exchange Avenue, Vaughan, ON, L4K 5R6 | trca.ca



From: Hamdy Shafi <<u>hshafi@cfcrozier.ca</u>>
Sent: September 18, 2024 10:26 AM
To: Michael Hynes <<u>Michael.Hynes@trca.ca</u>>; Dilnesaw Chekol <<u>Dilnesaw.Chekol@trca.ca</u>>
Cc: Heaven Lin <<u>hlin@cfcrozier.ca</u>>; Rebecca Archer <<u>rarcher@cfcrozier.ca</u>>; Mena Iskander <<u>miskander@cfcrozier.ca</u>>;
Canejo, Carlos <<u>ccanejo@prologis.com</u>>; Joe Plutino <<u>jplutino@mainlineplanning.com</u>>; Jim Davidson
<Jdavidson@mainlineplanning.com>
Subject: RE: Prologis Humber Station - SWM Outlet Discussion with TRCA

EXTERNAL SENDER

Good Morning Michael,

Hope all is well.

I just wanted to follow up on my email from last week. Hoping to get your feedback on the framework we have outlined below to confirm we have the correct understanding before getting into our work.

Thanks in advance for your assistance.

Regards, Hamdy

Hamdy Shafi, P.Eng. Manager, Land Development Office: 416.842.0022 Collingwood | Milton | Toronto | Bradford | Guelph

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From: Hamdy Shafi <<u>hshafi@cfcrozier.ca</u>>

Sent: Thursday, September 12, 2024 10:35 AM

To: Michael.hynes@trca.ca; Dilnesaw A. DAC. Chekol < dchekol@trca.on.ca>

Cc: Heaven Lin <<u>hlin@cfcrozier.ca</u>>; Rebecca Archer <<u>rarcher@cfcrozier.ca</u>>; Mena Iskander <<u>miskander@cfcrozier.ca</u>>; Canejo, Carlos <<u>ccanejo@prologis.com</u>>; Joe Plutino <<u>jplutino@mainlineplanning.com</u>>; Jim Davidson <<u>Jdavidson@mainlineplanning.com</u>>; SWMA Outlet Discussion with TBCA

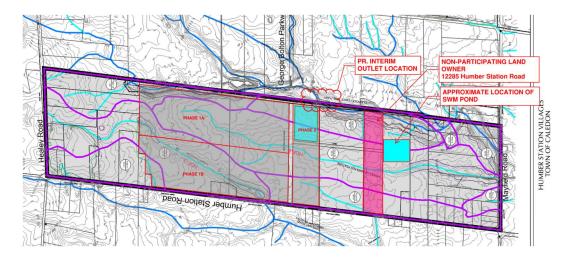
Subject: Prologis Humber Station - SWM Outlet Discussion with TRCA

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 - b. Compare the modeled flows from the diversion area to the modeled flows in the Clarkway Tributary.
- 2. Conduct flow comparison and sensitivity analysis in the VO model regarding flow diversion for the Clarkway Tributary.
 - a. Review and assess the downstream impact assuming the flow change and no SWM controls.
 - i. Assess the flow change within the Clarkway Tributary at the proposed interim outlet location.
 - ii. Assess the flow change within the Clarkway Tributary downstream of the proposed outlet to the confluence at node 43.2 (downstream of Mayfield Road).
 - b. Complete sensitivity analysis for the following:
 - i. Required Site release rate calculated based on unitary release rate calculated using the existing drainage area to the Clarkway Tributary.
 - ii. Unit flow rates calculated using the area for the existing areas draining to Clarkway Tributary and the area to be diverted from HDF 8.
 - iii. Proposed Site release rate to Clarkway Tributary (rate calculated in bullet ii reduced by the percentage required to produce no significant downstream impacts.
- 3. Conduct hydraulic downstream impact assessment.
 - a. Update the HEC-RAS model with the updated flow from task #2.
 - b. Evaluate the impact on water surface elevations and velocity between the proposed interim outlet location and node 43.2 (Mayfield Road).

The above tasks (1, 2, and 3) will be summarized in a memorandum for TRCA's review. Additional topics such as erosion and water balance criteria were also discussed in the meeting, and these criteria will be incorporated into the submission for Draft Plan of Subdivision, Zoning Bylaw Amendment, and Site Plan Application.

- 1. Erosion Assessment
 - a. Fluvial engineer to review the erosion impact at the outlet location and points of interest downstream of the Site.

- b. We will explore opportunities to increase onsite retention, such as 10 mm on-site retention will be provided on the roof and removed through evapotranspiration.
- c. 48-hour drawdown time is required for the SWM pond.
- 2. Water balance
 - a. Site specific water balance should be maintained, matching post-development infiltration volume to pre-development.
 - b. Fill loading impact on soil infiltration, mounding and LID infiltration rates should be reviewed by a geotechnical and hydrogeological engineer.

Please advise if our understanding of the above is correct. We look forward to providing more information to begin the TRCA's preliminary review and advancement of the Prologis Humber Station site design.

Thanks, Hamdy

Hamdy Shafi, P.Eng. Manager, Land Development Office: 416.842.0022 Collingwood | Milton | Toronto | Bradford | Guelph

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By: MJ Checked by: RA UC01 1.44

Hydrologic Parameters: NASHYD Command Pre-Development Drainage Area: Catchment UC01

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	1.44
Total Area Check				1.44

	Road	way	Sidew	valk	Gravel Par	rking Lot	Buildi	ng	SWM	٩F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	0.00	89			0.00						0.00
rvious Lan	duses Present:										
	Wood	land	Mear	MOM	Wetle	nd	Mead	0\W/S	Landscap	e/lawn	Subtoto
Soils	Wood Area (ha)	land CN	Meac Area (ha)	dow CN	Wetlc Area (ha)	and CN	Mead Area (ha)	ows CN	Landscap Area (ha)	e/Lawn CN	Subtoto Area
Soils MOG											
						CN			Area (ha)	CN	Area

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C	
Pervious	1.44	0.25	0.25	
Impervious	0.00	0.90	0.00	
Total Subcatchment	1.4	-	0.25	

Initial Abstraction Calculations

Landuse	IA (mm)	Area (ha)
Pervious	5	1.4
Impervious	1	0.00
Total	5.0	1.4

Time to Peak Calculations

	Tim	ne to Peak Inpi	uts			Uplands		Bransby	Williams	Air	oort
Length (m)	Drop (m)	Slope (%)	V/S ^{0.5}	Velocity (m/s)	Tc (hr)	Tp(hr)	TOTAL Tp (hr)	Tc (hr)	Tp(hr)	Tc (hr)	Tp(hr)
335	7.34	2.19%	2.3	0.34	0.27	0.16	0.16	0.26	0.18	0.65	0.44

Appropriate calculated time to peak: 0.44 Appropriate Method: Airport



By: MJ Checked by: RA UC02 0.98

Hydrologic Parameters: NASHYD Command Pre-Development Drainage Area: Catchment UC02

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	1.0
Total Area Check				1.0

	Roady	way	Sidew	valk	Gravel Par	rking Lot	Buildi	ng	SWN	٨F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	0.00	89			0.00						0.00
ervious Lan	duses Present:										
	Wood	and	Mead	wob	Wetlc	and	Mead	ows	Landscap	e/Lawn	Subtotal
Soils	Woodl Area (ha)	and CN	Meac Area (ha)	dow CN	Wetlc Area (ha)	and CN	Mead Area (ha)	ows CN	Landscap Area (ha)	e/Lawn CN	Subtotal Area
Soils MOG											
				CN		CN			Area (ha)	CN	Area

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C
Pervious	0.98	0.25	0.25
Impervious	0.00	0.90	0.00
Total Subcatchment	1.0	-	0.25

Initial Abstraction Calculations

Landuse	IA (mm)	Area (ha)		
Pervious	5	1.0		
Impervious	1	0.00		
Total	5.0	1.0		

Time to Peak Calculations

	Tin	ne to Peak Inpi	uts			Uplands		Bransby	Williams	Airp	oort
Length (m)	Drop (m)	Slope (%)	V/S ^{0.5}	Velocity (m/s)	Tc (hr)	Tp(hr)	TOTAL Tp (hr)	Tc (hr)	Tp(hr)	Tc (hr)	Tp(hr)
68	2.6	3.82%	2.3	0.45	0.04	0.03	0.03	0.05	0.03	0.24	0.16

Appropriate calculated time to peak:	0.16	Appropriate Method:	Airport
Minimum Tp = 0.17hr or 10 minutes is used in VO model		-	



By: MJ Checked by: RA UC03 0.45

Hydrologic Parameters: NASHYD Command Pre-Development Drainage Area: Catchment UC03

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	0.5
Total Area Check				0.5

	Roady	way	Sidev	valk	Gravel Par	rking Lot	Buildi	ing	SWA	٨F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	0.00	89			0.00						0.00
ərvious Lan	duses Present:										
	Wood	and	Mead	dow	Wetlo	and	Mead	ows	Landscap	e/Lawn	Subtota
Soils	Woodl Area (ha)	and CN	Meac Area (ha)	dow CN	Wetlc Area (ha)	and CN	Mead Area (ha)	ows CN	Landscap Area (ha)	e/Lawn CN	Subtota Area
Soils MOG											
				CN		CN			Area (ha)	CN	Area

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C
Pervious	0.45	0.25	0.25
Impervious	0.00	0.90	0.00
Total Subcatchment	0.5	-	0.25

Initial Abstraction Calculations

Landuse	IA (mm)	Area (ha)
Pervious	5	0.5
Impervious	1	0.00
Total	5.0	0.5

Time to Peak Calculations

	Airport	
Length (m) Drop (m) Slope (%) V/S ^{0.5} Velocity Tc (hr) Tp(hr) TOTAL Tp (hr) Tc (hr) Tp(hr)	Tc (hr) Tp(hr)	
75 1.75 2.33% 2.3 0.35 0.06 0.04 0.04 0.07 0.04	0.30 0.20	

Appropriate calculated time to peak: 0.20 Appropriate Method: Airport



By: MJ Checked by: RA

Hydrologic Parameters: STANDHYD Command Post-Development Drainage Area: Catchment C201

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	1.45
Total Area Check				1.45

	Paved/Re	ooftops	Sidew	valk	Parking	g Lot	Buildi	ng	SWN	٨F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	1.40	98									1.40
ervious Lan	duses Present:										
	Wood	land	Mead	dow	Wetlo	and	Mead	ows	Landscap	e/Lawn	Subtotal
Soils		land CN	Meac Area (ha)	dow CN	Wetlc Area (ha)	and CN	Mead Area (ha)	ows CN	Landscap Area (ha)	e/Lawn CN	Subtotal Area
	Wood										
Soils	Wood					CN			Area (ha)	CN	Area

Initial Abstraction Calculations

Landuse	IA (mm)	Area (ha)	
Pervious	5	0.052	
Impervious	1	1.40	
Total	1.1	1.45	

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C
Pervious	0.05	0.25	0.01
Impervious	1.40	0.90	0.87
Total Subcatchment	1.45	-	0.88

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	2	145	0.25
Impervious	2.0	1.1	100	0.013



By: MJ Checked by: RA C201R 2.40

Hydrologic Parameters: STANHYD Command Post-Development Drainage Area: Catchment C201R

Curve Number Calculation

ID	Hydrologic Group	% Area	Area
MOG	С	100	2.4
			2.4
	ID MOG	, , , ,	, , , , , , , , , , , , , , , , , , , ,

Impervious L	anduses Present										
	Paved/R	ooftops	Sidev	valk	Parkir	ng Lot	Build	ing	SWN	٩F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	2.40	98									2.40
Pervious Lan	ervious Landuses Present:										
	Wood	lland	Mead	wob	Wetl	and	Mead	ows	Landscap	e/Lawn	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG										74	0.00
					CN Calculation	_	Total Area				2.40
						5	Pervious Curve	Number			74

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C
Pervious	0.00	0.25	0.00
Impervious	2.40	0.90	0.90
Total Subcatchment	2.40	-	0.90

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	2	21	0.25
Impervious	1.0	1	21	0.013



By: MJ Checked by: RA

C202

Hydrologic Parameters: STANDHYD Command Post-Development Drainage Area: Catchment C202

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	1.7
Total Area Check				1.7

	Paved/Rc	oftops	Sidew	valk	Parking	g Lot	Buildi	ng	SWM	۱F	Subtota
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	1.70	98									1.70
rvious Lan	duses Present:										
	MALE A SH		11	1.	147 - 11 -		14		1	. //	
	Woodle	and	Meac	wob	Wetlo	Ind	Mead	ows	Landscap	e/Lawn	Subtote
Soils	Woodl Area (ha)	and CN	Meac Area (ha)	dow CN	Wetla Area (ha)	ind CN	Mead Area (ha)	ows CN	Landscap Area (ha)	e/Lawn CN	Subtot Area
Soils MOG											
						CN				CN	Area

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C		
Pervious	0.00	0.25	0.00		
Impervious	1.70	0.90	0.90	TIMP	0.99
Total Subcatchment	1.70	-	0.90	XIMP	0.99

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	2	40	0.25
Impervious	2.0	1.1	100	0.013



By: MJ Checked by: RA C202R 2.37

Hydrologic Parameters: STANDHYD Command Post-Development Drainage Area: Catchment C202R

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	2.37
Total Area Check				2.37

	Paved/R	ooftops	Sidew	valk	Parking	g Lot	Buildi	ng	SWN	١F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	2.37	98									2.37
ervious Lan	duses Present:										
										0	
	Wood	land	Meac	wob	Wetlo	and	Mead	ows	Landscap	e/Lawn	Subtotal
Soils	Wood Area (ha)	land CN	Meac Area (ha)	dow CN	Wetlc Area (ha)	and CN	Mead Area (ha)	ows CN	Landscap Area (ha)	e/Lawn CN	Subtotal Area
Soils MOG											
				CN		CN				CN	Area

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C		
Pervious	0.00	0.25	0.00		
Impervious	2.37	0.90	0.90	TIMP	
Total Subcatchment	2.37	-	0.90	XIMP	

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	2	21	0.25
Impervious	1.0	1	21	0.013



By: MJ Checked by: RA C203 1.33

Hydrologic Parameters: STANDHYD Command Post-Development Drainage Area: Catchment C203

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	1.3
Total Area Check				1.3

	Paved/Re	ooftops	Sidew	valk	Parking	g Lot	Buildi	ng	SWN	٩F	Subtota
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	1.31	98									1.31
ervious Lan	duses Present:										
	Wood	land	Maga	low	Wata	and	Magd	0.1.10	Landsoan	o /l m /m	Culatat
Soile	Wood		Meac		Wetlc		Mead		Landscap		Subtoto
Soils	Wood Area (ha)	land CN	Meac Area (ha)	dow CN	Wetlc Area (ha)	and CN	Mead Area (ha)	ows CN	Area (ha)	CN	Area
Soils MOG											
				CN		CN			Area (ha)	CN	Arec

Initial Abstraction Calculations

Landuse	IA (mm)	Area (ha)
Pervious	5	0.022
Impervious	1	1.31
Total	1.1	1.33

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C
Pervious	0.00	0.25	0.00
Impervious	1.31	0.90	0.90
Total Subcatchment	1.31	-	0.90

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	2	40	0.25
Impervious	2.0	1.1	100	0.013



By: MJ Checked by: RA C203R 2.40

Hydrologic Parameters: STANDHYD Command Post-Development Drainage Area: Catchment C203R

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	2.4
Total Area Check				2.4

	anduses Present: Paved/Ro		Sidev	valk	Parkir	ng Lot	Build	ing	SWM	٨F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	2.40	98									2.40
Pervious Lan	duses Present:										
	Woodl	and	Mead	wob	Wetle	and	Mead	ows	Landscap	e/Lawn	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG										74	0.00
							Total Area				2.40
					CN Calculation						

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C		
Pervious	0.00	0.25	0.00		
Impervious	2.40	0.90	0.90	TIMP	0.99
Total Subcatchment	2.40	-	0.90	XIMP	0.99

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	1	21	0.25
Impervious	1.0	1	21	0.013



By: MJ Checked by: RA

C204 1.61

Hydrologic Parameters: STANDHYD Command Post-Development Drainage Area: Catchment C204

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	1.6
Total Area Check				1.6

	Paved/Re	ooftops	Sidew	valk	Parking	g Lot	Buildi	ng	SWN	٨F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	1.48	98									1.48
ervious Lan	duses Present:										
01110000 2011											-
0111000 2011	Wood	land	Mead	wob	Wetlo	and	Mead	ows	Landscap	e/Lawn	Subtotal
Soils	Wood Area (ha)	land CN	Meac Area (ha)	dow CN	Wetla Area (ha)	and CN	Mead Area (ha)	ows CN	Landscap Area (ha)	e/Lawn CN	Subtotal Area
Soils						CN			Area (ha)	CN	Area

Initial Abstraction Calculations

Landuse	IA (mm)	Area (ha)
Pervious	5	0.126
Impervious	1	1.48
Total	1.3	1.61

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C
Pervious	0.00	0.25	0.00
Impervious	1.48	0.90	0.90
Total Subcatchment	1.48	-	0.90

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	2	145	0.25
Impervious	2.0	1.1	100	0.013



By: MJ Checked by: RA C204R 2.39

Hydrologic Parameters: STANDHYD Command Post-Development Drainage Area: Catchment C204R

Curve Number Calculation

ID	Hydrologic Group	% Area	Area
MOG	С	100	2.4
			2.4
	ID MOG	, , , ,	, , , , , , , , , , , , , , , , , , , ,

	anduses Present Paved/R		Sidev	valk	Parking	g Lot	Buildi	ing	SWN	ΛF	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	2.39	98									2.39
⁵ ervious Lan	iduses Present:										
	Wood	land	Mead	wob	Wetlo	and	Mead	ows	Landscap	e/Lawn	Subtotal
Soils	Wood Area (ha)	land CN	Meac Area (ha)	dow CN	Wetlc Area (ha)	and CN	Mead Area (ha)	ows CN	Landscap Area (ha)	e/Lawn CN	Subtotal Area
Soils MOG											
						CN				CN	Area

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C		
Pervious	0.00	0.25	0.00		
Impervious	2.39	0.90	0.90	TIMP	
Total Subcatchment	2.39	-	0.90	XIMP	

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	1	21	0.25
Impervious	1.0	1	21	0.013



By: MJ Checked by: RA C205 1.69

Hydrologic Parameters: STANDHYD Command Post-Development Drainage Area: Catchment C205

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	1.7
Total Area Check				1.7

	Paved/Re	ooftops	Sidew	valk	Parkin	g Lot	Build	ing	SWN	٨F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	1.69	98									1.69
'ervious Lan	duses Present:										
	Wood	land	Mead	wob	Wetlo	and	Mead	ows	Landscap	e/Lawn	Subtotal
Soils	Wood Area (ha)	land CN	Meac Area (ha)	dow CN	Wetla Area (ha)	and CN	Mead Area (ha)	ows CN	Landscap Area (ha)	e/Lawn CN	Subtotal Area
Soils MOG											
						CN				CN	Area

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C		
Pervious	0.00	0.25	0.00		
Impervious	1.69	0.90	0.90	TIMP	
Total Subcatchment	1.69	-	0.90	XIMP	

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	2	40	0.25
Impervious	2.0	1.1	100	0.013



By: MJ Checked by: RA C205R 2.36

Hydrologic Parameters: STANDHYD Command Post-Development Drainage Area: Catchment C205R

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	2.4
Total Area Check				2.4

	anduses Present: Paved/Ro		Sidev	valk	Parkin	ng Lot	Build	ing	SWA	٨F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	2.36	98									2.36
Pervious Lan	duses Present:										
	Woodl	and	Mead	wob	Wetle	and	Mead	lows	Landscap	e/Lawn	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG										74	0.00
					CNI Calaviation		Total Area				2.36
					CN Calculations	S	Pervious Curve	AL			74

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C		
Pervious	0.00	0.25	0.00		
Impervious	2.36	0.90	0.90	TIMP	
Total Subcatchment	2.36	-	0.90	XIMP	

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	1	21	0.25
Impervious	1.0	1	21	0.013



By: MJ Checked by: RA C206 1.32

Hydrologic Parameters: STANDHYD Command Post-Development Drainage Area: Catchment C206

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	1.3
Total Area Check				1.3

	Paved/Ro	oftops	Sidew	valk	Parking	g Lot	Buildi	ng	SWN	٨F	Subtoto
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	1.31	98									1.31
rvious Lan	duses Present: Wood	and	Meac	low	\\/otio	nd	Magd	0.1.12	Landsoan	o /l anun	C. Jakat
				10W	Wetlo	ana	Mead	OWS	Landscap	e/Lawn	Subtot
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
Soils MOG					Area (ha)	CN	Area (ha)			CN 74	Arec 0.01
				CN	Area (ha)		Area (ha)		Area (ha)	-	

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C	
Pervious	0.00	0.25	0.00	
Impervious	1.31	0.90	0.90	TIMP
Total Subcatchment	1.3	-	0.90	XIMP

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	2	40	0.25
Impervious	2.0	1.1	100	0.013



By: MJ Checked by: RA Catchment Name: Catchment Area (ha):

C206R 2.39

Hydrologic Parameters: STANDHYD Command Post-Development Drainage Area: Catchment C206R

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	2.4
Total Area Check				2.4

Impervious L	Paved/Ro		Sidew	valk	Parking	g Lot	Buildi	ng	SWN	۱F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	2.39	98									2.39
^s ervious Lan	duses Present:										
	Wood	land	Mead	NOF	Wetlo	ind	Mead				Subtotal
		iania	mout	1011	WORK	inu	Medu	0 44 5	Landscap	C/LUWII	30010101
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
Soils MOG	Area (ha)										
	Area (ha)			CN		CN				CN	Area

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C		
Pervious	0.00	0.25	0.00		
Impervious	2.39	0.90	0.90	TIMP	0.
Total Subcatchment	2.4	-	0.90	XIMP	0.

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	1	21	0.25
Impervious	1.0	1	21	0.013



By: MJ Checked by: RA

C207A

Hydrologic Parameters: STANDHYD Command Post-Development Drainage Area: Catchment C207A

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	1.2
Total Area Check				1.2

	Paved/Ro	ooftops	Sidew	valk	Parking	g Lot	Buildi	ng	SWN	٨F	Subtota
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	1.24	89									1.24
rvious Lan	duses Present:									0	
Soils	Woodl Area (ha)	and CN	Meac Area (ha)	dow CN	Wetla Area (ha)	IND CN	Mead Area (ha)	ows CN	Landscap Area (ha)	e/Lawn CN	Subtot Area
Soils MOG	Woodi Area (ha)		Meac Area (ha)				Mead Area (ha)		Landscap Area (ha) 0.08		
				CN		CN			Area (ha)	CN	Area

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C
Pervious	0.00	0.25	0.00
Impervious	1.24	0.90	0.90
Total Subcatchment	1.2	-	0.90

Initial Abstraction Calculations

Landuse	IA (mm)	Area (ha)
Pervious	5	0.084
Impervious	1	1.24
Total	1.3	1.32

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	2	40	0.25
Impervious	2.0	2.5	60	0.013



By: MJ Checked by: RA

C207B

Hydrologic Parameters: STANDHYD Command Post-Development Drainage Area: Catchment C207B

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	1.4
Total Area Check				1.4

Impervious L	anduses Present. Paved/Ro		Sidev	valk	Parking	g Lot	Buildi	ing	SWN	٨F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	1.41	89									1.41
Pervious Lan	duses Present:										
	11/				147 11					()	
	Wood		Mead		Wetlo		Mead		Landscap		Subtotal
Soils	Wood Area (ha)	and CN	Meac Area (ha)	dow CN	Wetlc Area (ha)	and CN	Mead Area (ha)	ows CN	Landscap Area (ha)	e/Lawn CN	Subtotal Area
Soils MOG											
				CN		CN			Area (ha)	CN	Area

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C		
Pervious	0.00	0.25	0.00		
Impervious	1.41	0.90	0.90	TIMP	0.93
Total Subcatchment	1.4	-	0.90	XIMP	0.93

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	2	40	0.25
Impervious	2.0	2.5	60	0.013



By: MJ Checked by: RA

Hydrologic Parameters: STANDHYD Command Pre-Development Drainage Area: Catchment C208A

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	1.5
Total Area Check				1.5

	Paved/Re	ooftops	Sidew	valk	Parking	g Lot	Buildi	ng	SWN	٩F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	1.08	98									1.08
ervious Lan	duses Present:										
	Wood	land	Mead	dow	Wetlo	and	Mead	ows	Landscap	e/Lawn	Subtotal
Soils		land CN	Meac Area (ha)	dow CN	Wetlc Area (ha)	and CN	Mead Area (ha)	ows CN	Landscap Area (ha)	e/Lawn CN	Subtotal Area
	Wood										
Soils	Wood					CN			Area (ha)	CN	Area

Initial Abstraction Calculations

Landuse	IA (mm)	Area (ha)
Pervious	5	0.370
Impervious	1	1.08
Total	2.0	1.45

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C
Pervious	0.00	0.25	0.00
Impervious	1.08	0.90	0.90
Total Subcatchment	1.1	-	0.90

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	2	40	0.25
Impervious	2.0	2	AUTO	0.013



By: MJ Checked by: RA C208B 0.59

Hydrologic Parameters: STANDHYD Command Pre-Development Drainage Area: Catchment C208B

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	0.6
Total Area Check				0.6

	Roadway/R	looftops	Sidew	/alk	Parking	g Lot	Buildi	ng	SWN	٨F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	0.59	89									0.59
rvious Lan	iduses Present:										
			Mana				Manal			a /l. au . ua	1 C 1. 1. 1.
	Woodle		Meac		Wetlo		Mead		Landscap		Subtoto
Soils		and CN	Meac Area (ha)	low CN	Wetlc Area (ha)	and CN	Mead Area (ha)	ows CN	Landscap Area (ha)	e/Lawn CN	Subtoto Area
	Woodle										
Soils	Woodle			CN		CN			Area (ha)	CN	Area

TIMP XIMP

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C
Pervious	0.00	0.25	0.00
Impervious	0.59	0.90	0.90
Total Subcatchment	0.6	-	0.90

- 1	0.69
	0.69

Initial Abstraction Calculations

Landuse	IA (mm)	Area (ha)
Pervious	5	0.261
Impervious	1	0.59
Total	2.2	0.85

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	2	40	0.25
Impervious	1.0	2	AUTO	0.013



By: MJ Checked by: RA

Hydrologic Parameters: STANDHYD Command Pre-Development Drainage Area: Catchment C209

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	11.4
Total Area Check				11.4

	Roadway/R	ooftops	Sidew	valk	Parking	g Lot	Buildi	ng	SWM	٨F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	11.38	89									11.38
rvious Lan	duses Present:										
	Woodle	and	Meac	wok	Wetlo	ind	Mead	ows	Landscap	e/Lawn	Subtoto
Soils	Woodle Area (ha)	and CN	Meac Area (ha)	dow CN	Wetla Area (ha)	ind CN	Mead Area (ha)	ows CN	Landscap Area (ha)	e/Lawn CN	Subtoto Area
Soils MOG											
						CN				CN	Area

TIMP

XIMP

0.99

0.99

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C
Pervious	0.00	0.25	0.00
Impervious	11.38	0.90	0.90
Total Subcatchment	11.4	-	0.90

Initial Abstraction	Calculations
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Landuse	IA (mm)	Area (ha)
Pervious	5	0.000
Impervious	1	11.38
Total	1.0	11.38

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	2	145	0.25
Impervious	1.0	2	AUTO	0.013



By: MJ Checked by: RA

Hydrologic Parameters: STANDHYD Command Pre-Development Drainage Area: Catchment C210

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	10.9
Total Area Check				10.9

	Roadway/R	looftops	Sidew	valk	Parking	g Lot	Buildi	ng	SWN	۱F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	10.90	89									10.90
rvious Lan	duses Present:										
	Woodle	and	Meac	wob	Wetlo	and	Mead	ows	Landscap	e/Lawn	Subtoto
Soils	Woodlo Area (ha)	and CN	Meac Area (ha)	dow CN	Wetlc Area (ha)	and CN	Mead Area (ha)	ows CN	Landscap Area (ha)	e/Lawn CN	Subtoto Area
Soils MOG											
						CN	Area (ha)	CN			Area

TIMP XIMP

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C
Pervious	0.00	0.25	0.00
Impervious	10.90	0.90	0.90
Total Subcatchment	10.9	-	0.90

0.99
0.99

Initial Abstraction Calculations

Landuse	IA (mm)	Area (ha)
Pervious	5	0.000
Impervious	1	10.90
Total	1.0	10.90

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	2	145	0.25
Impervious	1.0	2	AUTO	0.013



By: MJ Checked by: RA

Hydrologic Parameters: STANDHYD Command Post-Development Drainage Area: Catchment C201

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	С	100	26.06
Total Area Check				26.06

	anduses Present: Paved/Rc	oftops	Sidew	valk	Parking	g Lot	Buildi	ng	SWN	۱F	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	25.85	98									25.85
Pervious Lar	nduses Present:										
	Woodl	and	Meac	wob	Wetla	ind	Mead	ows	Landscap	e/Lawn	Subtotal
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
Soils MOG	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha) 0.32	CN 74	0.32
	Area (ha)	CN	Area (ha)		Area (ha)		Area (ha) Total Area	CN		-	

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C
Pervious	0.32	0.25	0.00
Impervious	25.85	0.90	0.89
Total Subcatchment	26.16	-	0.89

Land Use	IA (mm)	Slope (%)	Travel Length (m)	Manning's n
Pervious	5.0	2	145	0.25
Impervious	2.0	1.1	100	0.013



Project Name: Prologis Humber Station Project Number: 624-6777 Date: 2024.04.11

By: MJ Checked by: RA

Hydrologic Parameters: NASHYD Command Pre-Development Drainage Area: Catchment C103

Curve Number Calculation

Soil Types Present:				
Туре	ID	Hydrologic Group	% Area	Area
Monogham Clay Loam	MOG	D	100	3.8
Total Area Check				3.8

	Roadv	vay	Sidew	valk	Gravel Par	rking Lot	Buildi	ng	SWM	٨F	Subtota
Soils	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area (ha)	CN	Area
MOG	0.00	89			0.00						0.00
rvious Lan	duses Present:)\/	ue el		-		- La - d	
	Woodle		Mead		Wetla		Law		Cultivo		Subtote
Soils	Woodl Area (ha)	and CN	Meac Area (ha)	JOW CN	Area (ha)	CN	Area (ha)	n CN	Area (ha)	CN	Area
Soils MOG											
				CN		CN			Area (ha)	CN	Area

Runoff Coefficient Calculations

Land Use	Area (ha)	с	Weighted Average C
Pervious	3.77	0.25	0.25
Impervious	0.00	0.90	0.00
Total Subcatchment	3.8	-	0.25

Initial Abstraction Calculations

Landuse	IA (mm)	Area (ha)	
Pervious	5	3.8	
Impervious	1	0.00	
Total	5.0	3.8	

Time to Peak Calculations

	Tim	ne to Peak Inpu	uts			Uplands		Bransby	Williams	Airp	oort
Length (m)	Drop (m)	Slope (%)	V/S ^{0.5}	Velocity (m/s)	Tc (hr)	Tp(hr)	TOTAL Tp (hr)	Tc (hr)	Tp(hr)	Tc (hr)	Tp(hr)
530	5.34	1.01%	2.3	0.23	0.64	0.38	0.38	0.44	0.29	1.06	0.71

Appropriate calculated time to peak: 0.71 Appropriate Method: Airport



Humber River Unitary Flow Rates Summary - Phase 1A - Interim Conditions Outlet to Clarkway Drive Tributary

Clarway Trib Area =	9.56	ha					
Humber River Wate	Humber River Watershed Sub-Basin 36						
Return Period	Controlled Release						
Reform Felloa	Rate (L/s/ha)						
2-Year	7.9						
5-Year	12.1						
10-Year	14.9						
25-Year	18.7						
50-Year	21.9						
100-Year	24.7						

NOTE:

Q - unit flow (L/s/ha - litres per second per hectare)
 A - area in hectares (ha).

3) Pre-development unit flow rate area

4) Equation: 29.912-2.316*LN(Area)

Sub-Basin ID	36				
Return Period	Calculated Unit Flow Rate				
Kelolitrellou	L/s	(m ³ /s)			
2-Year	75.4	0.075			
5-Year	115.6	0.116			
10-Year	142.0	0.142			
25-Year	178.9	0.179			
50-Year	209.0	0.209			
100-Year	236.0	0.236			



Humber River Unitary Flow Rates Summary - Phase 1A - Ultimate Conditions Outlet to SWM Pond 3

CEISMP Area	64.22	ha			
Humber River Watershed Sub-Basin 36					
Return Period	Controlled Release Rate (L/s/ha)				
2-Year	6.51				
5-Year	9.92				
10-Year	12.24				
25-Year	15.39				
50-Year	17.90				
100-Year	20.27				

NOTE:

1) Q - unit flow (L/s/ha - litres per second per hectare)

2) A - area in hectares (ha).

3) Pre-development unit flow rate area

4) Equation: 29.912-2.316*LN(Area)

5) The controlled release rates are calculated with 64.2 ha, and consistent with Table 4.10 in the CEISMP Phase 2 Report (Schaeffers, August 2024).

Sub-Basin ID	36			
Existing Contributing Area	29.52	ha		
Return Period	Calculated Unit Flow Rate			
kelom renod	L/s	(m ³ /s)		
2-Year	192.3	0.192		
5-Year	292.9	0.293		
10-Year	361.4	0.361		
25-Year	454.3	0.454		
50-Year	528.4	0.528		
100-Year	598.4	0.598		

NOTE:

1) Existing contributing areas are based on Drawing C120: Pre-development Drainage Plan.

Regional Flow Criteria

Humber River Catchment ID	Colour (Outlet)	Regional Release Rate (L/s/ha)		
43.10	Blue (HDF-6)	136.0		
43.03	Orange (HDF-8)	71.0		
43.06	Green (HDF-14)	102.5		

Description	Existing Contributing Area (ha)	Humber River Catchment ID	Colour (Outlet)	Regional Release Rate (L/s/ha)	Total Regional Release Rate (m ³ /s)
	1.47	43.10	Blue (HDF-6)	200.3	
Phase 1A	26.92	43.03	Orange (HDF-8)	1911.5	2.227
	1.12	43.06	Green (HDF-14)	115.0	
Street A	1.04	43.03	Orange (HDF-8)	73.8	0.112
SlieerA	0.37	43.06	Green (HDF-14)	37.9	0.112

NOTE:

1) Street A will be George Bolton Parkway in the ultimate condition.



Diverted Area Changes

CATCHMENT ID	AREA (ha)	DIVERTED SITE CATCHMENTS	DIVERTED SITE CATCHMENT AREA (ha)	AREA W/O DIVERTED AREA (ha)
41.07	101.08	UC02	0.15	96.10
		C201	0.10	
		C201R	0.40	
		C204	1.61	
		C204R	1.97	
		C205	0.69	
		C205R	0.05	
	TOTAL		4.98	
43.10	202.72	UC01	0.49	194.79
		UC02	0.83	
		C201	1.35	
		C201R	2.00	
		C202	1.70	
		C202R	0.55	
		C203	0.81	
		C203R	0.22	
	TOTAL		7.94	
43.06	35.79	UC01	0.95	33.31
		C203	0.52	
		C203R	0.20	
		C207B	0.80	
	TOTAL		2.47	
43.03	63.04	C202R	1.82	49.11
		C203R	1.98	
		C204	0.00	
		C204R	0.42	
		C205	1.00	
		C205R	2.31	
		C206	1.32	
		C206R	2.39	
		C207A	1.24	
		C208A	1.45	
	TOTAL		13.93	



J124

ADDHYD	Storm Event	Scenario	Phase 1A Release Rate (m3/s)	Phase 1A Storage (ha.m)	Pre-d	ev Peak Flow (m3/s)	Post-de	v Peak Flo	w (m3/s)	Perce	ent Increas	e (%)
			(110/3)	(na.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR
		Uncontrolled	2.490	0.000				10.531	10.531	9.471	0.4	0.9	-0.5
J124	2 YEAR	Unit Flow Rate	0.075	0.980	10.489	10.438	9.514	10.214	10.170	9.294	-2.6	-2.6	-2.3
J124	ZILAN	Unit Flow Rate x 4	0.300	0.705	10.407	10.450	7.514	10.450	10.408	9.511	-0.4	-0.3	0.0
		Proposed Solution	0.192	0.652				10.311	10.273	9.390	-1.7	-1.6	-1.3

J4200.683

ADDHYD	Storm Event	Scenario	Phase 1A Release Rate	Phase 1A Storage	Pre-d	ev Peak Flow (m3/s)	Post-de	v Peak Flo	w (m3/s)	Perce	ent Increas	e (%)
			(m3/s)	(ha.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR
		Uncontrolled	2.490	0.000				10.651	11.185	10.514	0.3	0.5	0.4
J4200.683	2 YEAR	Unit Flow Rate	0.075	0.980	10.618	11.133	10.470	10.407	10.919	10.278	-2.0	-1.9	-1.8
J4200.003	ZILAN	Unit Flow Rate x 4	0.300	0.705	10.010	11.155	10.470	10.641	11.157	10.497	0.2	0.2	0.3
		Proposed Solution	0.192	0.652				10.505	11.019	10.383	-1.1	-1.0	-0.8

J1700.594

ADDHYD	Storm Event	Scenario	Phase 1A Release Rate	Phase 1A Storage	Pre-d	ev Peak Flow (m3/s)	Post-de	v Peak Flo	w (m3/s)	Perce	ent Increas	e (%)
	-		(m3/s)	(ha.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR
		Uncontrolled	2.490	0.000				10.589	11.571	11.177	0.8	0.6	0.5
J1700.594	2 YFAR	Unit Flow Rate	0.075	0.980	10.510	11.502	11.118	10.368	11.341	10.967	-1.4	-1.4	-1.4
31700.374	ZILAN	Unit Flow Rate x 4	0.300	0.705	10.510	11.502	11.110	10.556	11.546	11.157	0.4	0.4	0.4
		Proposed Solution	0.192	0.652				10.448	11.437	11.070	-0.6	-0.6	-0.4

<u>Scenarios:</u>

Uncontrolled Unit Flow Rate Unit Flow Rate x 4 Proposed Solution Divereted all flows from Phase 1A without controls Phase 1A controlled to unitary flow rates Phase 1A controlled to unitary flow rates x 4 Phase 1A controlled to CEISMP requirements



J124

A	DDHYD	Storm Event	Scenario	Phase 1A Release Rate (m3/s)	Phase 1A Storage (ha.m)	Pre-de	ev Peak Flow (m3/s)	Post-de	v Peak Flo	w (m3/s)	Perce	ent Increas	e (%)
				(110/3)	(na.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR
			Uncontrolled	3.332	0.000				16.891	15.995	13.552	0.2	0.6	-1.0
	1124	5 YEAR	Unit Flow Rate	0.116	1.227	16.855	15.901	13.683	16.456	15.540	13.340	-2.4	-2.3	-2.5
	J124	JILAK	Unit Flow Rate x 4	0.464	0.921	16.655	13.701	13.005	16.823	15.887	13.646	-0.2	-0.1	-0.3
J124 5 Y		Proposed Solution	0.293	0.808				16.592	15.665	13.469	-1.6	-1.5	-1.6	

J4200.683

ADDHYD	Storm Event	Scenario	Phase 1A Release Rate	Phase 1A Storage	Pre-de	ev Peak Flow (m3/s)	Post-de	v Peak Flo	w (m3/s)	Perce	ent Increas	e (%)
			(m3/s)	(ha.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR
		Uncontrolled	3.33	0.000				17.253	17.048	15.175	0.4	0.2	-0.1
J4200.683	5 YEAR	Unit Flow Rate	0.116	1.227	17,191	17.012	15,193	16.856	16.683	14.865	-1.9	-1.9	-2.2
J4200.663	JIEAK	Unit Flow Rate x 4	0.464	0.921	17.171	17.012	15.175	17.223	17.038	15.185	0.2	0.2	-0.1
		Proposed Solution	0.293	0.808				16.998	16.818	15.004	-1.1	-1.1	-1.2

J1700.594

ADDHYD	Storm Event	Scenario	Phase 1A Release Rate	Phase 1A Storage	Pre-de	ev Peak Flow (m3/s)	Post-de	v Peak Flo	w (m3/s)	Perce	ent Increas	e (%)
			(m3/s)	(ha.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR
		Uncontrolled	3.33	0.000				16.289	16.968	15.705	0.7	0.5	0.4
J1700.594	5 YEAR	Unit Flow Rate	0.116	1.227	16,180	16.876	15.643	15.957	16.640	15.402	-1.4	-1.4	-1.5
J1700.374	JILAK	Unit Flow Rate x 4	0.464	0.921	10.100	10.070	15.645	16.227	16.924	15.671	0.3	0.3	0.2
		Proposed Solution	0.293	0.808				16.075	16.761	15.536	-0.6	-0.7	-0.7

Scenarios: Uncontrolled Unit Flow Rate

Divereted all flows from Phase 1A without controls Phase 1A controlled to unitary flow rates Unit Flow Rate x 4 Phase 1A controlled to unitary flow rates x 4

Proposed Solution Phase 1A controlled to CEISMP requirements



J124

ADDHYD	Storm Event	Scenario	Phase 1A Release Rate (m3/s)	Phase 1A Storage (ha.m)	Pre-de	ev Peak Flow (m3/s)	Post-de	v Peak Flo	w (m3/s)	Perce	ent Increas	e (%)
			(110/3)	(na.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR
		Uncontrolled	3.899	0.000				21.385	19.930	16.395	0.0	0.3	-0.7
1124	10 YEAR	Unit Flow Rate	0.142	1.430	21.382	19.869	16.517	20.865	19.399	16.153	-2.4	-2.4	-2.2
J124	TUTLAK	Unit Flow Rate x 4	0.568	1.071	21.302	17.007	10.517	21.314	19.817	16.518	-0.3	-0.3	0.0
J124 10		Proposed Solution	0.361	0.882				21.053	19.567	16.312	-1.5	-1.5	-1.2

J4200.683

ADDHYD	Storm Event	Scenario	Phase 1 A Release Rate	Phase 1A Storage	Pre-de	ev Peak Flow (m3/s)	Post-de	v Peak Flo	ow (m3/s)	Perce	ent Increas	e (%)
			(m3/s)	(ha.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR
		Uncontrolled	3.899	0.000				21.767	21.147	18.437	0.5	0.3	-0.2
J4200.683	10 YEAR	Unit Flow Rate	0.142	1.430	21.659	21.079	18,479	21.249	20.682	18.099	-1.9	-1.9	-2.1
J4200.003	TOTLAK	Unit Flow Rate x 4	0.568	1.071	21.007	21.077	10.477	21.714	21.118	18.475	0.3	0.2	0.0
		Proposed Solution	0.361	0.882				21.444	20.862	18.269	-1.0	-1.0	-1.1

J1700.594

ADDHYD	Storm Event	Scenario	Phase 1A Release Rate		Pre-de	ev Peak Flow (m3/s)	Post-de	v Peak Flo	w (m3/s)	Perce	ent Increas	e (%)
			(m3/s)	(ha.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR
		Uncontrolled	3.899	0.000				20.214	20.755	18.814	0.8	0.6	0.3
J1700.594	10 YEAR	Unit Flow Rate	0.142	1.430	20.046	20.626	18.766	19.744	20.305	18.480	-1.5	-1.6	-1.5
51700.574	TOTLAK	Unit Flow Rate x 4	0.568	1.071	20.040	20.020	10.700	20.120	20.694	18.771	0.4	0.3	0.0
		Proposed Solution	0.361	0.882				19.912	20.478	18.624	-0.7	-0.7	-0.8

<u>Scenarios:</u> Uncontrolled Unit Flow Rate

Divereted all flows from Phase 1A without controls Phase 1A controlled to unitary flow rates Phase 1A controlled to unitary flow rates x 4

 Unit Flow Rate x 4
 Phase 1A controlled to unitary flow rates x 4

 Proposed Solution
 Phase 1A controlled to CEISMP requirements



J124

ADDHY	ADDHYD Storm Event	Scenario	Phase 1A Release Rate (m3/s)	Phase 1A Storage (ha.m)	Pre-d	ev Peak Flow	(m3/s)	Post-de	v Peak Flo	w (m3/s)	Perce	ent Increas	.e (%)
			(110/3)	(na.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR
		Uncontrolled	4.617	0.000				28.374	25.269	20.268	0.2	-0.1	-0.9
1124	25 VEAD	Unit Flow Rate	0.179	1.666	28.307	25.297	20,462	27.304	24.644	19.992	-3.5	-2.6	-2.3
J124	23 TLAK	Unit Flow Rate x 4	0.716	1.248	20.307	23.277	20.402	27.877	25.162	20.439	-1.5	-0.5	-0.1
		Proposed Solution	0.454	0.977				27.588	24.894	20.191	-2.5	-1.6	-1.3

J4200.683

ADDHYD	Storm Event	Scenario	Phase 1 A Release Rate	Phase 1A Storage	Pre-d	ev Peak Flow ((m3/s)	Post-de	v Peak Flo	w (m3/s)	Perce	ent Increas	e (%)
			(m3/s)	(ha.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR
		Uncontrolled	4.62	0.000				27.711	26.707	22.835	1.0	1.0	-0.1
14200 493	25 YEAR	Unit Flow Rate	0.179	1.666	27,434	26.436	22.857	26.925	25.994	22.399	-1.9	-1.7	-2.0
J4200.003	ZUTLAK	Unit Flow Rate x 4	0.716	1.248	27.434	20.430	22.007	27.523	26.557	22.874	0.3	0.5	0.1
ADDHYD		Proposed Solution	0.454	0.977				27.202	26.253	22.616	-0.8	-0.7	-1.1

J1700.594

ADDHYD	Storm Event	Scenario	Phase 1A Release Rate	Phase 1A Storage	Pre-d	ev Peak Flow ((m3/s)	Post-de	v Peak Flo	w (m3/s)	Perce	ent Increas	e (%)
			(m3/s)	(ha.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR
		Uncontrolled	4.62	0.000				25.563	25.788	23.174	0.8	0.7	0.6
J1700.594	25 YEAR	Unit Flow Rate	0.179	1.666	25.359	25.603	23.045	25.005	25.244	22.698	-1.4	-1.4	-1.5
51700.574	20 ILAK	Unit Flow Rate x 4	0.716	1.248	23.337	25.005	23.043	25.465	25.703	23.113	0.4	0.4	0.3
		Proposed Solution	0.454	0.977				25.218	25.461	22.895	-0.6	-0.6	-0.7

<u>Scenarios:</u>

Uncontrolled Divereted all flow Unit Flow Rate Phase 1A contro Unit Flow Rate x 4 Phase 1A contro Proposed Solution Phase 1A contro

Divereted all flows from Phase 1A without controls Phase 1A controlled to unitary flow rates Phase 1A controlled to unitary flow rates x 4

Phase 1A controlled to CEISMP requirements



J124

ADDHYD Storm Event		Scenario	Phase 1A Phase 1A Release Rate Storage (m3/s) (ha.m)		Pre-de	v Peak Flow	(m3/s)	Post-dev Peak Flow (m3/s)			Percent Increase (%)		
			(1113/3)	(na.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR
		Uncontrolled	5.156	0.000	33.397			33.503	29.820	23.421	0.3	0.3	-0.9
1124		Unit Flow Rate	0.209	1.837		29,734	23.623	32.624	29.124	23.125	-2.3	-2.1	-2.1
	Unit Flow Rate x 4	0.836	1.378	55.577	27.734	23.025	33.293	29.723	23.633	-0.3	0.0	0.0	
	Proposed Solution	0.528	1.019				32.952	29.420	23.352	-1.3	-1.1	-1.1	

J4200.683

ADDHYD Storm Event	Scenario	Phase 1 A Release Rate	Phase 1 A Storage	Pre-dev	v Peak Flow	(m3/s)	Post-de	v Peak Flo	w (m3/s)	Perce	ent Increas	e (%)	
			(m3/s)	(ha.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR
	14000 (92 - 50 YEAR	Uncontrolled	5.16	0.000		30.984	26.431	33.913	31.217	26.373	0.7	0.8	-0.2
14200 483		Unit Flow Rate	0.209	1.837	33.683			32.936	30.169	25.901	-2.2	-2.6	-2.0
J4200.683 50 YEAR	JUTEAK	Unit Flow Rate x 4	0.836	1.378	55.665			33.573	30.957	26.454	-0.3	-0.1	0.1
	Proposed Solution	0.528	1.019				33.253	30.569	26.155	-1.3	-1.3	-1.0	

J1700.594

ADDHYD Storm Event	Scenario	Phase 1 A Release Rate		Pre-dev	v Peak Flow	(m3/s)	Post-de	v Peak Flo	w (m3/s)	Perce	ent Increas	e (%)	
			(m3/s)	(ha.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR
		Uncontrolled	5.16	0.000		29.269	26.261	29.736	29.501	26.409	0.8	0.8	0.6
11700 594	50 YEAR	Unit Flow Rate	0.209	1.837	29,487			29.023	28.864	25.871	-1.6	-1.4	-1.5
J1700.594 5	JUTEAK	Unit Flow Rate x 4	0.836	1.378	27.40/		20.201	29.603	29.403	26.349	0.4	0.5	0.3
		Proposed Solution	0.53	1.019				29.305	29.115	26.093	-0.6	-0.5	-0.6

<u>Scenarios:</u>

Uncontrolled Unit Flow Rate

Divereted all flows from Phase 1A without controls Phase 1A controlled to unitary flow rates Unit Flow Rate x 4 Phase 1A controlled to unitary flow rates x 4

Proposed Solution Phase 1A controlled to CEISMP requriements



J124

ADDHYD Storm Event		Scenario	Phase 1A Release Rate (m3/s)	Phase 1A Storage (ha.m)	Pre-dev	v Peak Flow	v (m3/s)	Post-de	v Peak Flo	w (m3/s)	Perce	ent Increas	e (%)
		kule (mo/s)	(na.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	
	Uncontrolled	5.693	0.000				39.106	34.202	26.918	-0.3	0.2	-1.1	
J124	100 YEAR	Unit Flow Rate	0.236	2.014	39.218	34,135	27.208	38.351	33.452	26.591	-2.2	-2.0	-2.3
JI24 IOU YEAR	TOUTLAK	Unit Flow Rate x 4	0.944	1.513	37.210	34.135	27.208	39.105	34.122	27.162	-0.3	0.0	-0.2
	Proposed Solution	0.598	1.064				38.707	33.775	26.853	-1.3	-1.1	-1.3	

J4200.683

ADDHYD Storm Event		Scenario	Phase 1A Release	Phase 1A Storage	Pre-dev	/ Peak Flow	/ (m3/s)	Post-de	v Peak Flo	w (m3/s)	Perc	ent Increas	e (%)
		Rate (m3/s)	(ha.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	
		Uncontrolled	5.69	0.000		35.964	30.467	39.739	36.131	30.346	0.8	0.5	-0.4
J4200.683	100 YEAR	Unit Flow Rate	0.236	2.014	39.415			38.657	35.285	29.799	-1.9	-1.9	-2.2
J4200.683 TO	TOU TEAK	Unit Flow Rate x 4	0.944	1.513	37.413		50.407	39.403	35.934	30.459	0.0	-0.1	0.0
		Proposed Solution	0.598	1.064				39.008	35.594	30.108	-1.0	-1.0	-1.2

J1700.594

ADDHYD Storm Event		Scenario	Phase 1 A Release	Phase 1A Storage	Pre-dev	Peak Flow	/ (m3/s)	Post-de	ev Peak Flo	w (m3/s)	Perce	ent Increas	e (%)
		Rate (m3/s)	(ha.m)	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	6 HOUR	12 HOUR	24 HOUR	
		Uncontrolled	5.69	0.000		33.286	29.516	34.558	33.611	29.697	1.0	1.0	0.6
11700 594	100 YEAR	Unit Flow Rate	0.236	2.014	34.201			33.547	32.727	29.088	-1.9	-1.7	-1.5
J1700.594 10	TOUTLAK	Unit Flow Rate x 4	0.944	1.513	34.201		27.510	34.305	33.414	29.620	0.3	0.4	0.4
		Proposed Solution	0.598	1.064				33.911	33.043	29.338	-0.8	-0.7	-0.6

<u>Scenarios:</u>

Uncontrolled Unit Flow Rate Unit Flow Rate x 4 Proposed Solution Divereted all flows from Phase 1A without controls Phase 1A controlled to unitary flow rates Phase 1A controlled to unitary flow rates x 4 Phase 1A controlled to CEISMP requirements



Regional Results

J124							
ADDHYD	Storm Event	Scenario	Phase 1A Release Rate (m3/s)	Phase 1A Storage (ha.m)	Pre-dev Peak Flow (m3/s)	Post-dev Peak Flow (m3/s)	Percent Increase (%)
		Uncontrolled	4.303	0.000		77.841	2.2
1124	Hazel1000	Unit Flow Rate	0.679	4.965	76.143	75.200	-1.2
J124 H	Huzen 000	Unit Flow Rate x 2.5	1.697	3.362	70.143	76.101	-0.1
		CEISMP	3.362	1.654		75.769	-0.5

J4200.683

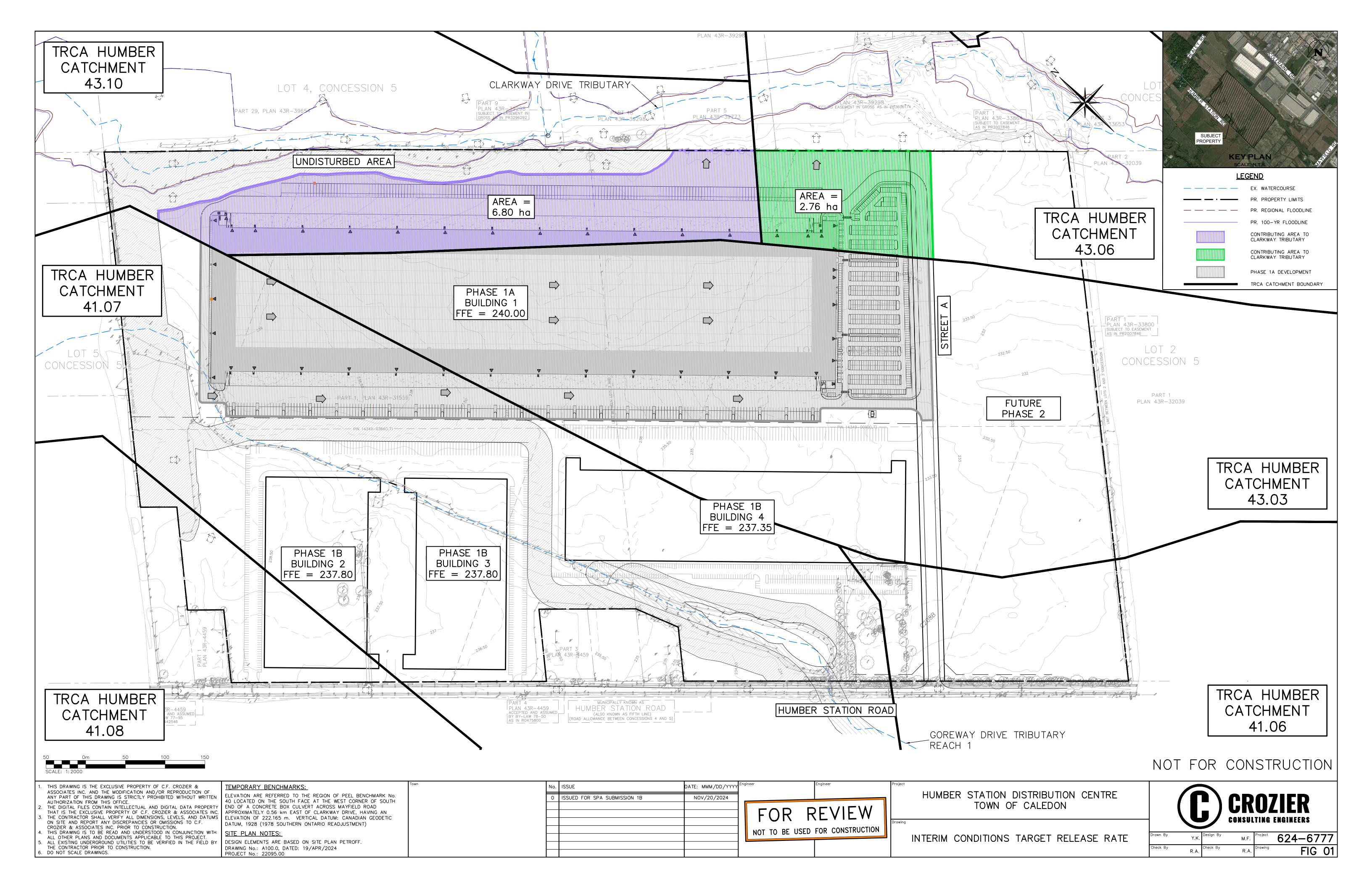
ADDHYD	Storm Event	Scenario	Phase 1A Release Rate (m3/s)	Phase 1A Storage (ha.m)	Pre-dev Peak Flow (m3/s)	Post-dev Peak Flow (m3/s)	Percent Increase (%)
		Uncontrolled	4.303	0.000		92.357	2.3
J4200.683	Hazel1000	Unit Flow Rate	0.679	4.965	00 200	89.299	-1.1
J4200.663	падентооо	Unit Flow Rate x 2.5	1.697	3.362 90.288		90.246	0.0
		CEISMP	3.362	1.654		89.917	-0.4

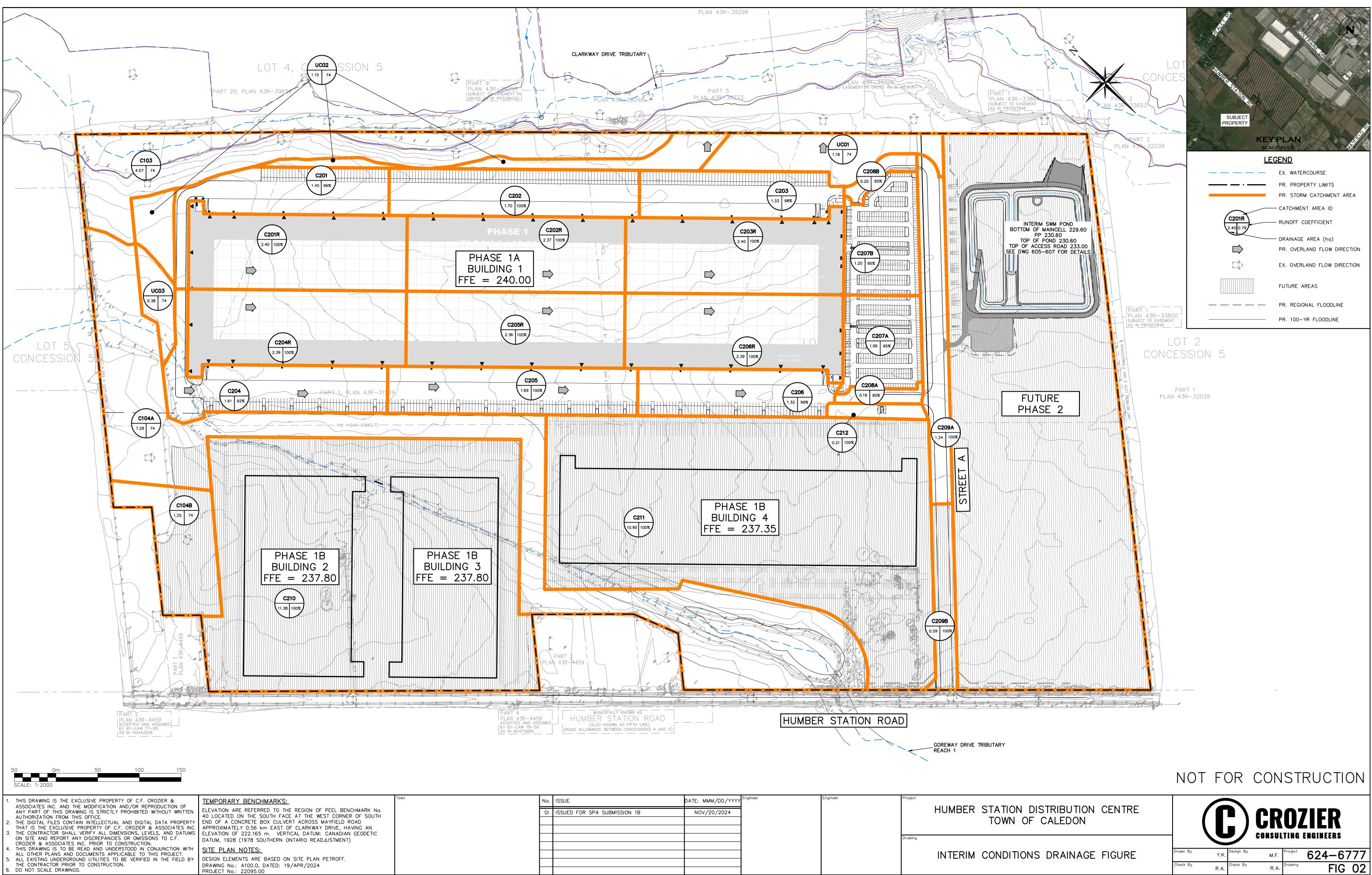
J1700.594

ADDHYD	Storm Event	Scenario	Phase 1A Release Rate (m3/s)	Phase 1A Storage (ha.m)	Pre-dev Peak Flow (m3/s)	Post-dev Peak Flow (m3/s)	Percent Increase (%)
		Uncontrolled	4.303	0.000		112.683	1.9
11700 594	J1700.594 Hazel1000	Unit Flow Rate	0.679	4.965	110.616	109.800	-0.7
51700.074		Unit Flow Rate x 2.5	1.697	3.362	110.010	110.594	0.0
		CEISMP	3.362	1.654		110.332	-0.3

Scenarios:

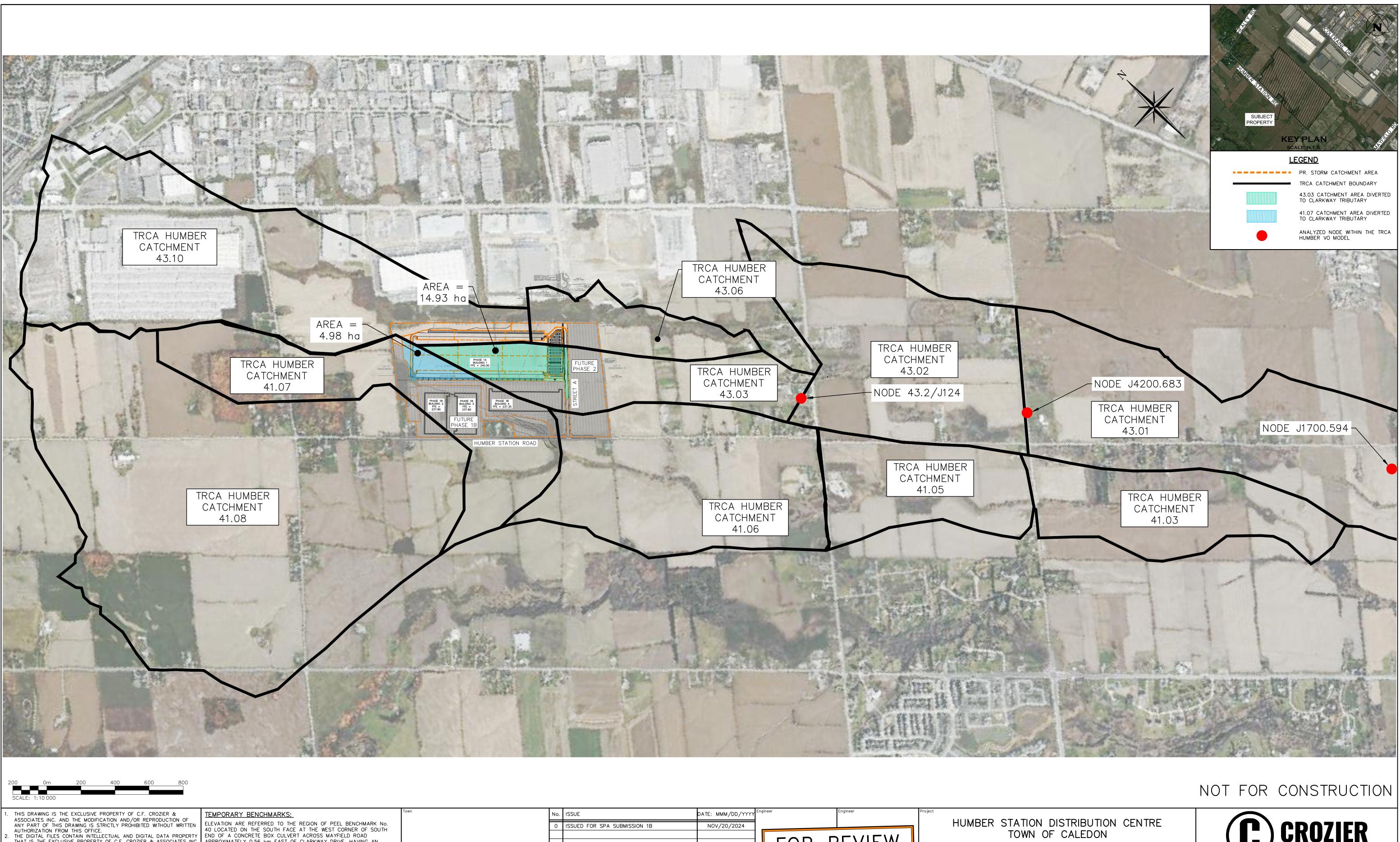
Uncontrolled Unit Flow Rate Unit Flow Rate x 2.5 CEISMP Divereted all flows from Phase 1A without controls Phase 1A controlled to regional unitary flow rates Phase 1A controlled to regional unitary flow rates x 2.5 Phase 1A controlled to CEISMP requirements





No.	ISSUE	DATE: MMM/DD/YYYY	Engineer	Engineer	Project		
.03	ISSUED FOR SPA SUBMISSION 1B	NOV/20/2024				HUMBER	STATION TOWN O
					Drawing		
						INTERIM	CONDITIO

FIG 02 R.A. R.A.



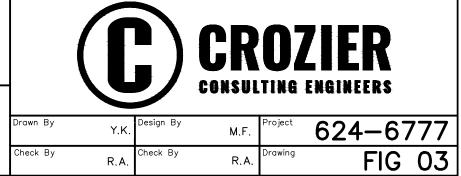
2.	THE DIGITAL FILES CONTAIN INTELLECTUAL AND DIGITAL DATA PROPER
	THAT IS THE EXCLUSIVE PROPERTY OF C.F. CROZIER & ASSOCIATES
3.	THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, LEVELS, AND DATU
	ON SITE AND REPORT ANY DISCREPANCIES OR OMISSIONS TO C.F.
	CROZIER & ASSOCIATES INC. PRIOR TO CONSTRUCTION.
A	THE DRAWING IS TO BE DEAD AND UNDERSTOOD IN CONJUNCTION W

. THIS DRAWING IS TO BE READ AND UNDERSTOOD IN CONJUNCTION WITH ALL OTHER PLANS AND DOCUMENTS APPLICABLE TO THIS PROJECT. . ALL EXISTING UNDERGROUND UTILITIES TO BE VERIFIED IN THE FIELD BY DESIGN ELEMENTS ARE BASED ON SITE PLAN PETROFF. THE CONTRACTOR PRIOR TO CONSTRUCTION. 6. DO NOT SCALE DRAWINGS.

APPROXIMATELY 0.56 km EAST OF CLARKWAY DRIVE, HAVING AN ELEVATION OF 222.165 m. VERTICAL DATUM: CANADIAN GEODETIC DATUM, 1928 (1978 SOUTHERN ONTARIO READJUSTMENT)

DRAWING No.: A100.0, DATED: 19/APR/2024 PROJECT No.: 22095.00

No	o. ISSUE	DATE: MMM/DD/YYY	Y ^{Engi}	jineer	Engineer	Project			
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						Drawing			
			+	NOT TO BE US	ED FOR CONSTRUCTION			TIONS	DI



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